

STORM WATER MANAGEMENT CALCULATIONS - QUALITY TREATMENTS

WATERMARK on the BAY SUBDIVISION

COUNTY of PRINCE EDWARD (Village of Rossmore)

CHECK DAMS

Check dams are proposed to be installed in the road side ditches.

Volume of storage behind check dams

Bottom Width (m)	0.5	0.5	0.5	0.75
Side Slopes	3:1	3:1	3:1	2.5:1
Depth of Storage (m)	0.3	0.3	0.3	0.3
Ditch Slope	0.55%	0.50%	1.00%	4.40%
Volume (m ³)	7.6	8.4	4.2	1.0
Number Provided	2	6	2	1
Volume (m ³)	15.3	50.4	8.4	1.0
Total (m ³)	74.1			

CONSTRUCTED WETLAND

Smaller Drainage Area are viable where there is high groundwater table

Based on Table 3.2 Stormwater Management Planning and Design Manual

Enhanced Protection Level (80% Long-Term S.S. Removal)

For Impervious Level of 35% Required Storage Volume would be 80 m³/ha

Proposed Subdivision is calculated to be 12% Impervious

Extrapolating, Required Storage Volume would be 50 m³/ha

Subcatchment Area	9.69 ha
Required Storage Volume	484.5 m ³
Storage Collected Behind Check Dams	74.1 m ³
	410.4 m ³

VEGETATED FILTER STRIP

Discharge from Constructed Wetland to be with a Vegetated Filter Strip

$$Q = KLH^{1.5}$$

Q = Flow (m³/s)

L = Length (m)

H = Head of Water (m)

K = Constant

$$K = 2.3$$

$$L = 29 \quad \text{m}$$

Storm Event

	Quality	5 _{yr}	100 _{yr}
Q =	0.113	1.712	3.349
Depth of Flow over Filter Strip (mm) =	14.2	87.0	136.1
Velocity (m/s) =	0.3	0.7	0.8

DITCH CAPACITY CALCULATION SHEET

5yr STORM EVENT (Q=2.78 AIR, I= 25.4^{1+0.087})

POST-DEVELOPMENT CONDITIONS

STREET	FROM	TO	AREAS (ha)		Indiv. 2.76AR	Accum. 2.76AR	Time of Conc. min.	Rainfall Intensity I(mm/hr)	Peak Flow Q(l/s)	DITCH DATA											
			R=20	R=9						BOTTOM WIDTH	SIDE SLOPES	FLOW DEPTH	WETTED PERIMETER	FLOW AREA	SLOPE (%)	LENGTH (m)	CAPACITY (l/s) n= 0.030	VELOCITY (m/s)	TIME of FLOW (min)		
			RIGHT	LEFT																	
LEASIDE STREET	0+025	0+210	3.222	0.258	2.436	2.436	55.62	26	64.4	0.5	3	3	0.141	1.392	0.130	0.55%	185	66.3 OK	0.51	6.05	
	0+025	0+210	0.348	0.153	0.575	0.575	28.26	41	23.6	0.5	3	3	0.083	1.025	0.062	0.55%	185	23.7 OK	0.38	8.08	
NAVIGATION DRIVE	0+182	0+330	1.036	0.214	1.111	1.111	40.30	33	36.7	0.5	3	3	0.107	1.177	0.088	0.50%	148	36.7 OK	0.42	5.90	
	0+182	0+330	0.231	0.099	0.376	0.376	11.13	80	30.0	0.5	3	3	0.097	1.113	0.077	0.50%	148	30.4 OK	0.40	6.23	
	0+330	0+635	3.017	0.406	2.693	6.241	61.67	25	153.7	0.5	3	3	0.184	1.664	0.194	1.00%	305	153.8 OK	0.79	6.40	
	0+330	0+635	0.400	0.240	0.823	1.199	17.36	59	70.6	0.5	3	3	0.126	1.297	0.111	1.00%	305	71.5 OK	0.65	7.87	
	Into Weiland					7.440	68.07	23	171.2												
				1.663	0.417	1.968	1.968	45.65	30	59.6	0.5	3	3	0.137	1.366	0.125	0.50%	182	59.7 OK	0.48	6.35
201	0+182	0+005	0.256	0.134	0.478	0.478	11.62	78	37.1	0.5	3	3	0.077	0.987	0.056	0.50%	182	19.7 Surch	0.35	8.69	
EAST OUTLET	0+030	0+040			2.446	2.446	52.00	28	67.7	0.75	2.5	2.5	0.058	1.062	0.052	8.80%	10	68.6 OK	1.32	0.13	
	0+040	0+110			2.446	2.446	52.12	28	67.6	0.75	2.5	2.5	0.071	1.132	0.066	4.40%	70	68.1 OK	1.05	1.11	

DITCH CAPACITY CALCULATION SHEET

100yr STORM EVENT (Q=2.78 AIR, I= 39.7^{0.705})

POST-DEVELOPMENT CONDITIONS

STREET	FROM	TO	AREAS (ha)		Indiv. 2.78AR	Accum. 2.78AR	Time of Conc. min.	Rainfall Intensity I(mm/hr)	Peak Flow Q(l/s)	DITCH DATA						TIME of FLOW (min)				
			R=20	R=9						BOTTOM WIDTH	SIDE SLOPES RIGHT	SIDE SLOPES LEFT	FLOW DEPTH	WETTED PERIMETER	FLOW AREA		SLOPE (%)	LENGTH (m)	CAPACITY (l/s) n= 0.030	VELOCITY (m/s)
LEASIDE STREET	0+025	0+210	3.222	0.258	2.920	2.920	52.34	44	127.6	0.5	3	3	0.195	1.733	0.212	0.55%	185	128.7	0.61	5.07
	0+025	0+210	0.348	0.153	0.644	0.644	11.85	124	79.9	0.5	3	3	0.155	1.480	0.150	0.55%	185	80.2	0.54	5.75
NAVIGATION DRIVE	0+182	0+330	1.036	0.214	1.285	1.285	37.72	55	70.7	0.5	3	3	0.150	1.449	0.143	0.50%	148	71.6	0.50	4.91
	0+182	0+330	0.231	0.099	0.422	0.422	11.13	130	54.7	0.5	3	3	0.132	1.395	0.118	0.50%	148	55.4	0.47	5.27
105	0+330	0+635	3.017	0.406	3.169	7.374	57.41	41	302.0	0.5	3	3	0.253	2.100	0.319	1.00%	305	302.0	0.95	5.36
	0+330	0+635	0.400	0.240	0.912	1.334	16.40	99	131.7	0.5	3	3	0.171	1.581	0.173	1.00%	305	132.2	0.76	6.66
Into Wetland	0+182	0+005	1.663	0.417	2.257	2.257	15.07	105	236.4	0.5	3	3	0.266	2.182	0.345	0.50%	182	238.1	0.69	4.40
	0+182	0+005	0.256	0.134	0.532	0.532	11.62	126	66.9	0.5	3	3	0.146	1.423	0.137	0.50%	182	67.8	0.49	6.13
EAST OUTLET DITCH	0+030	0+040			2.790	2.790	19.47	87	244.0	0.75	2.5	2.5	0.119	1.391	0.125	8.80%	10	246.9	1.98	0.08
	0+040	0+110			2.790	2.790	19.55	87	243.3	0.75	2.5	2.5	0.143	1.520	0.158	4.40%	70	245.2	1.55	0.75

Tc CALCULATIONS

WATERMARK on the BAY SUBDIVISION
 VILLAGE of ROSSMORE
 COUNTY of PRINCE EDWARD (Ameliasburgh Ward)
 1470757 ONTARIO INC. - Owner

POST DEVELOPMENT CONDITION

Subcatchment 101

<u>Runoff Coefficient</u>	<u>5yr Storm Event</u>		<u>100yr Storm Event</u>	
C =	0.252	0.2	0.45	0.9
	3.480	3.222	0.258	0.258
	0.876	0.644	0.232	0.232

Airport Formula (C values less than 0.4)

Elev_H = 87.25
 Elev_L = 80.70
 A (ha) = 3.480
 C = 0.252
 L = 490
 S_w = 1.34%
 T_c = 55.62
 I = 26
 Q_p = 64.42

Elev_H = 87.25
 Elev_L = 80.70
 A (ha) = 3.480
 C = 0.302
 L = 490
 S_w = 0.013367
 T_c = 52.34
 I = 44
 Q_p = 127.60

Subcatchment 102

<u>Runoff Coefficient</u>	<u>5yr Storm Event</u>		<u>100yr Storm Event</u>	
C =	0.414	0.2	0.45	0.9
	0.500	0.348	0.153	0.153
	0.207	0.070	0.137	0.137

Airport Formula (C values less than 0.4)

Elev_H = 83.30
 Elev_L = 80.60
 A (ha) = 0.500
 C = 0.414
 L = 205
 S_w = 1.32%
 T_c = 29.26
 I = 41
 Q_p = 23.63

Elev_H = 83.30
 Elev_L = 80.60
 A (ha) = 0.500
 C = 0.464
 L = 205
 S_w = 1.32%
 T_c = 11.85
 I = 124
 Q_p = 79.86

(C values greater than 0.4)

Tc CALCULATIONS
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POST DEVELOPMENT CONDITION

Subcatchment 103

<u>Runoff Coefficient</u>	<u>5yr Storm Event</u>		<u>100yr Storm Event</u>	
C =	0.320	0.2	0.45	0.9
	1.250	1.036	0.214	0.214
	0.400	0.207	0.193	0.203

Airport Formula (C values less than 0.4)

Elev _H =	82.40	Elev _H =	82.40
Elev _L =	80.70	Elev _L =	80.70
A (ha) =	1.250	A (ha) =	1.250
C =	0.320	C =	0.370
L =	215	L =	215
S _w =	0.79%	S _w =	0.79%
T _c =	40.30	T _c =	37.72
I =	33	I =	55
Q _p =	36.66	Q _p =	70.67

Subcatchment 104

<u>Runoff Coefficient</u>	<u>5yr Storm Event</u>		<u>100yr Storm Event</u>	
C =	0.410	0.2	0.45	0.9
	0.330	0.231	0.099	0.099
	0.135	0.046	0.089	0.089

Bransby-Williams Formula (C values greater than 0.4)

Elev _H =	81.55	Elev _H =	81.55
Elev _L =	80.70	Elev _L =	80.70
A (ha) =	0.330	A (ha) =	0.330
C =	0.410	C =	0.460
L =	155	L =	155
S _w =	0.55%	S _w =	0.55%
T _c =	11.13	T _c =	11.13
I =	80	I =	130
Q _p =	30.04	Q _p =	54.66

(C values greater than 0.4)

Tc CALCULATIONS

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POST DEVELOPMENT CONDITION

Subcatchment 105

<u>Runoff Coefficient</u>	<u>5yr Storm Event</u>	<u>Runoff Coefficient</u>	<u>100yr Storm Event</u>
C =	0.295 0.2 0.45 0.9	C =	0.345 0.2 0.45
	3.490 3.017 0.473		3.490 3.017 0.473
	1.029 0.603 0.426		1.204 0.754 0.450

Airport Formula (C values less than 0.4)

Elev _H =	83.30	Elev _H =	83.30
Elev _L =	77.75	Elev _L =	77.75
A (ha)=	3.490	A (ha)=	3.490
C =	0.295	C =	0.345
L =	385	L =	385
S _w =	1.44%	S _w =	1.44%
T _c =	45.64	T _c =	42.81
I =	30	I =	50
Q _p =	86.68	Q _p =	168.41

Subcatchment 106

<u>Runoff Coefficient</u>	<u>5yr Storm Event</u>	<u>Runoff Coefficient</u>	<u>100yr Storm Event</u>
C =	0.463 0.2 0.45 0.9	C =	0.513 0.2 0.45
	0.640 0.400 0.240		0.640 0.400 0.240
	0.296 0.080 0.216		0.328 0.100 0.228

Bransby-Williams Formula (C values greater than 0.4)

Elev _H =	80.75	Elev _H =	80.75
Elev _L =	77.75	Elev _L =	77.75
A (ha)=	0.640	A (ha)=	0.640
C =	0.463	C =	0.513
L =	290	L =	290
S _w =	1.03%	S _w =	1.03%
T _c =	17.17	T _c =	17.17
I =	59	I =	96
Q _p =	48.82	Q _p =	87.17

(C values greater than 0.4)

Tc CALCULATIONS

WATERMARK on the BAY SUBDIVISION
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POST DEVELOPMENT CONDITION

Subcatchment 201

<u>Runoff Coefficient</u>	<u>5yr Storm Event</u>		<u>Runoff Coefficient</u>	<u>100yr Storm Event</u>	
C =	0.340	0.2	0.45	0.9	0.417
	2.080	1.663		0.390	0.45
	0.708	0.333		2.080	1.663
				0.812	0.416
					0.396

Airport Formula (C values less than 0.4)

Elev_H = 82.20
 Elev_L = 80.55
 A (ha) = 2.080
 C = 0.340
 L = 255
 S_w = 0.65%
 T_c = 45.65
 I = 30
 Q_p = 59.61

Bransby-Williams Formula (C values greater than 0.4)

Elev_H = 82.20
 Elev_L = 80.55
 A (ha) = 1.663
 C = 0.200
 L = 255
 S_w = 0.65%
 T_c = 15.07
 I = 105
 Q_p = 96.81

Subcatchment 202

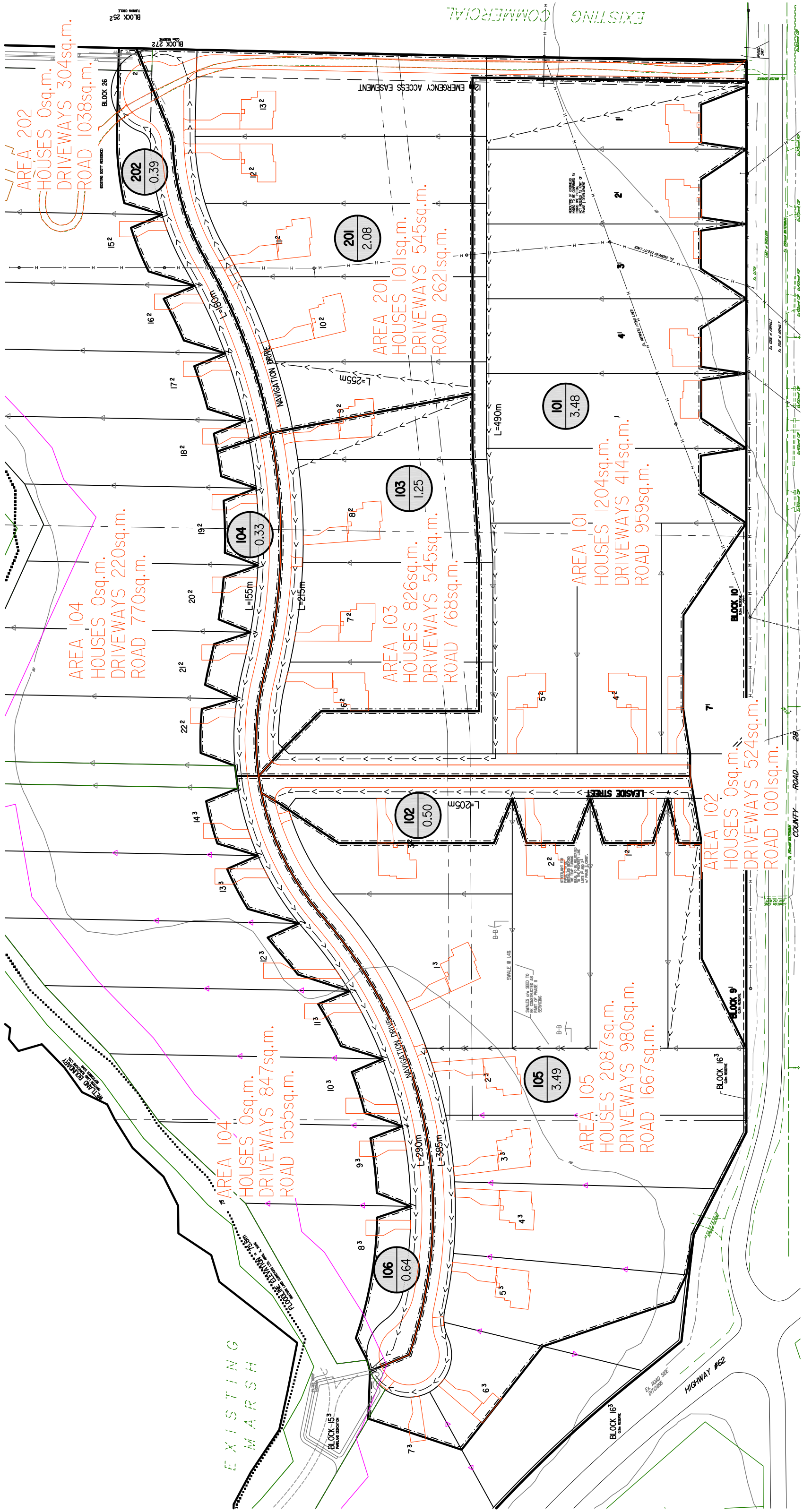
<u>Runoff Coefficient</u>	<u>5yr Storm Event</u>		<u>Runoff Coefficient</u>	<u>100yr Storm Event</u>	
C =	0.441	0.2	0.45	0.9	0.134
	0.390	0.256		0.390	0.134
	0.172	0.051		0.191	0.064
					0.127

Airport Formula (C values greater than 0.4)

Elev_H = 81.55
 Elev_L = 80.00
 A (ha) = 0.390
 C = 0.441
 L = 180
 S_w = 0.86%
 T_c = 11.62
 I = 78
 Q_p = 37.07

Bransby-Williams Formula (C values greater than 0.4)

Elev_H = 81.55
 Elev_L = 80.00
 A (ha) = 0.390
 C = 0.491
 L = 180
 S_w = 0.86%
 T_c = 11.62
 I = 126
 Q_p = 66.91



AREA 202
HOUSES 0sq.m.
DRIVEWAYS 304sq.m.
ROAD 1038sq.m.

AREA 104
HOUSES 0sq.m.
DRIVEWAYS 220sq.m.
ROAD 770sq.m.

AREA 201
HOUSES 101sq.m.
DRIVEWAYS 545sq.m.
ROAD 262sq.m.

AREA 103
HOUSES 826sq.m.
DRIVEWAYS 545sq.m.
ROAD 768sq.m.

AREA 101
HOUSES 1204sq.m.
DRIVEWAYS 414sq.m.
ROAD 959sq.m.

AREA 102
HOUSES 0sq.m.
DRIVEWAYS 524sq.m.
ROAD 1001sq.m.

AREA 104
HOUSES 0sq.m.
DRIVEWAYS 847sq.m.
ROAD 1555sq.m.

AREA 105
HOUSES 2087sq.m.
DRIVEWAYS 980sq.m.
ROAD 1667sq.m.

AREA 106
HOUSES 0sq.m.
DRIVEWAYS 847sq.m.
ROAD 1555sq.m.

DITCH CALCULATION PLAN
SECONDARY INFORMATION
DRAWING 58770-Si2

SCALE: 1:1600
DESIGNED: A.H.V.
DRAWN: S.D.S.
DATE: JUNE 2014
COMPUTER: \\Meer_drawing\drawings\Brien Parkside Landscaping\587-70_Watermark on the Bay\

DATE	DESCRIPTION	By	Chk'd By

WATERMARK BY THE BAY
VILLAGE OF ROSSMORE
1470757 ONTARIO INC. - OWNER

VAN MEER LIMITED
LAND DEVELOPMENT · PROJECT MANAGEMENT · ENGINEERING
14 Bridge Street West, Belleville, Ont. K8P 1H7
Tel. 613-969-0171 Fax. 613-969-1781
E-mail Address: meer_drawing@on.abn.com