

Cosmopolitan Homes (49 & 57 Folkard Lane) Servicing Report

September 2026

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1 BACKGROUND

The Greer Galloway Group has been asked to provide a Servicing Design Report for the proposed Cosmopolitan Homes development in Prince Edward County. The subject property is located on County Road 49, approximately 2.1 km north of its intersection with Bridge Street. The proposed development includes a single parcel of land totaling 3.09 hectares (7.6 acres).

The subject lands are currently vacant and zoned Urban Residential Type 1 (R1-44). There are no lands within the subject property that are classified as environmentally protected (EP).

This document will speak to the anticipated drinking water and sanitary servicing demands and the ability for the adjacent urban infrastructure to accommodate those demands in support of the proposed development. Additionally, this document will provide preliminary grading, sanitary design and watermain layout at the project location.

Stormwater, traffic and other development related concerns will be addressed separately under separate cover.

The proposed development will incorporate 98 residential units including a combination of bungalow townhouses and 3-storey townhouses. There is a total of 18 townhouse blocks proposed as part of this development. There is also the potential of a future commercial development in the northwest corner of the site. The exact distribution of the units is shown in the below table:

SITE AREA:	31 893.79 m ²	100%
NUMBER OF UNITS:		<u>98 UNITS</u>
3 STOREY REAR GARAGE TOWNHOUSES, TYPE 'A'		25 UNITS
BUNGALOW TOWNHOUSES WITH LOFT, TYPE 'B'		8 UNITS
3 STOREY TOWNHOUSES, TYPE 'C'		65 UNITS
R4 - 98 UNITS	MINIMUM LOT AREA • 145.28 m ²	
	<u>SITE AREA</u> 30660.31 m ²	100%
<u>COVERAGE</u>		
BLOCK 1, 3 STOREY REAR GARAGE TOWNHOUSES, TYPE 'A', 5 UNITS		374.07 m ²
BLOCK 2, 3 STOREY REAR GARAGE TOWNHOUSES, TYPE 'A', 4 UNITS		300.14 m ²
BLOCK 3, 3 STOREY REAR GARAGE TOWNHOUSES, TYPE 'A', 6 UNITS		447.68 m ²
BLOCK 4, 3 STOREY REAR GARAGE TOWNHOUSES, TYPE 'A', 4 UNITS		300.14 m ²
BLOCK 5, 3 STOREY REAR GARAGE TOWNHOUSES, TYPE 'A', 6 UNITS		447.68 m ²
BLOCK 6, 3 STOREY TOWNHOUSES, TYPE 'C', 4 UNITS		310.99 m ²
BLOCK 7, 3 STOREY TOWNHOUSES, TYPE 'C', 4 UNITS		310.99 m ²
BLOCK 8, 3 STOREY TOWNHOUSES, TYPE 'C', 5 UNITS		387.71 m ²
BLOCK 9, 3 STOREY TOWNHOUSES, TYPE 'C', 7 UNITS		541.15 m ²
BLOCK 10, 3 STOREY TOWNHOUSES, TYPE 'C', 7 UNITS		541.15 m ²
BLOCK 11, 3 STOREY TOWNHOUSES, TYPE 'C', 7 UNITS		541.15 m ²
BLOCK 12, BUNGALOW TOWNHOUSES WITH LOFT, TYPE 'B', 4 UNITS		484.10 m ²
BLOCK 13, BUNGALOW TOWNHOUSES WITH LOFT, TYPE 'B', 4 UNITS		484.10 m ²
BLOCK 14, 3 STOREY TOWNHOUSES, TYPE 'C', 6 UNITS		464.43 m ²
BLOCK 15, 3 STOREY TOWNHOUSES, TYPE 'C', 7 UNITS		541.15 m ²
BLOCK 16, 3 STOREY TOWNHOUSES, TYPE 'C', 6 UNITS		464.43 m ²
BLOCK 17, 3 STOREY TOWNHOUSES, TYPE 'C', 6 UNITS		464.43 m ²
BLOCK 18, 3 STOREY TOWNHOUSES, TYPE 'C', 6 UNITS		464.43 m ²
BLOCKS 1-18	7 869.92 m ²	
COVERED PORCH	350,2343 m ²	

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This total quantity and distribution of styles may vary somewhat as the development moves through the design and approval process, but minor variations are not likely to affect the conclusions made herein.

A series of images are provided to illustrate various elements of the subject lands.

Image 1: Property location relative to the surrounding community.



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Image 2: Property Zoning (Prince Edward County Secondary Plan)



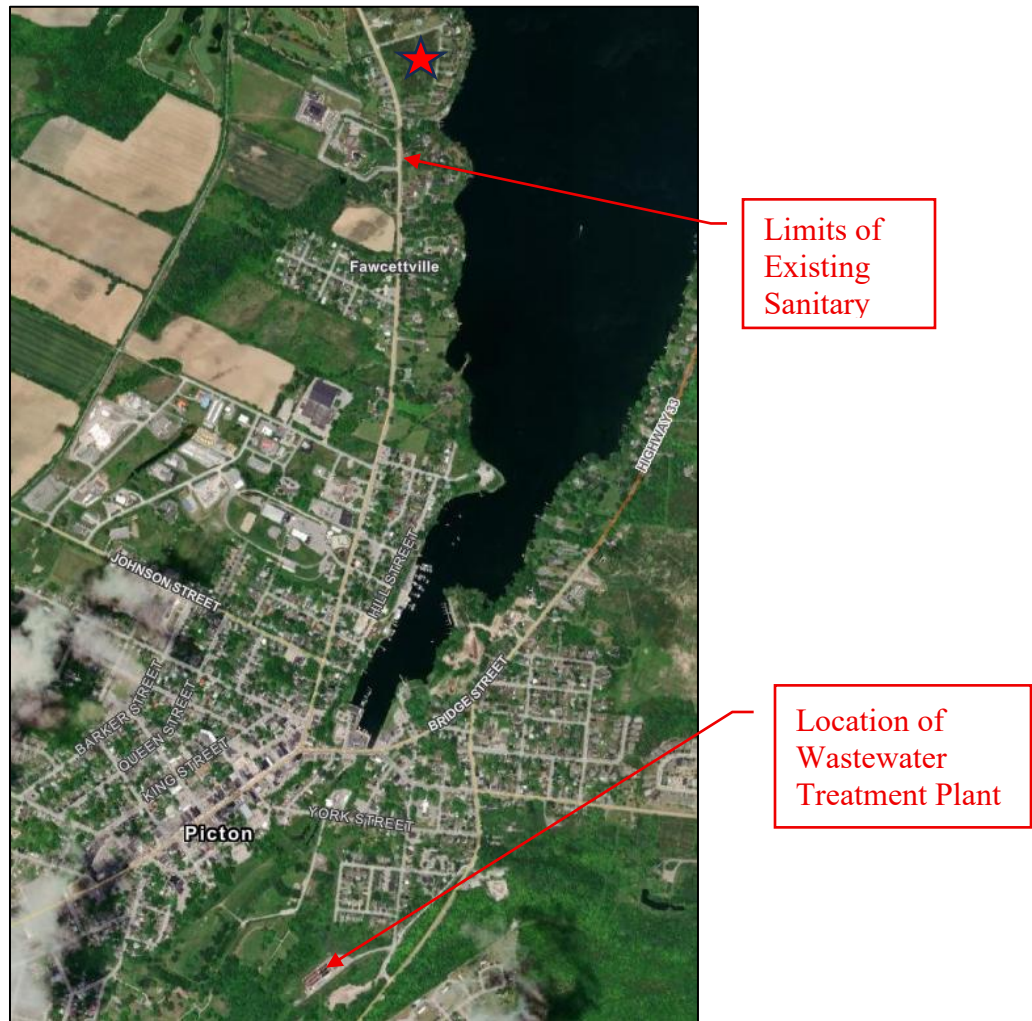
Image 3: Development Concept Plan



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2 WASTEWATER CONVEYANCE AND TREATMENT

While the subject lands are not currently serviced by the existing Town of Picton wastewater collection system and treatment plant, they are in the urban boundary and included in the future service area.



The development property lands are shown by the star in the image above. The existing sewage collection system extends along Picton Main Street to just north of Finnegan Drive. All properties north of this area are presumed to be serviced by existing septic systems. It is the understanding of Greer Galloway that the County intends to extend the existing sanitary system to just north of the subject property as part of an upcoming infrastructure project. This expansion to the existing collection network will be required and will result in additional treatment loads at the wastewater treatment plant.

In the existing system, flows are collected and conveyed via a combination of gravity networks and pumping stations and ultimately directed to the Picton Wastewater Treatment Plant west of Church / Pitt Street. Based on our preliminary review of the existing system, it is anticipated that the new sanitary sewer could be constructed in a manner that would permit the required Folkard Lane sanitary system to be conveyed via a gravity system. This is discussed in further detail in Section 2.2.

2.1 WASTEWATER TREATMENT PLANT

The Picton Wastewater Treatment Plant (WWTP) is located at 60 Church Street on the south side of Picton.

We have reviewed the documents available and provide the following summary of information:

Picton Master Servicing Plan (2024) – Wastewater Findings (October 2nd 2024 Public Meeting):

- Serves 2,515 people.
- Rated capacity of the plant is 6,000 m³/d.
- Sustainable plant capacity is 4,800 m³/d.

2023 Picton Wastewater Treatment Plant Annual Performance Report:

1. MECP Identifier Number: 120000667
2. Certificate of Approval #5464-AKATW7
3. Approved Capacity (Rated Capacity / Annual Average Daily Flow): 6,000 m³/day
4. Approved Capacity (Peak Daily Flow Rate): 26,400 m³/day
5. Flow records are summarized in the following report excerpts.

Capacity Assessment: Influent and Effluent Quantities

Based on the 2023 flow data, the Picton Wastewater Treatment Plant maintained operations within approved capacity requirements as per Environmental Compliance Approval No. 5464-AKATW7.

Table 11: Influent Quantity; Flow Data, 2023

Month	Approved Capacity (Rated Capacity) ¹	Monthly Average	Approved Capacity (Peak Daily Flow Rate) ²	Monthly Peak Flow
	m ³ /day	m ³ /day	m ³ /day	m ³ /day
January	6000	2402.90	26400	2899.08
February	6000	3694.28	26400	8700.72
March	6000	4448.13	26400	6242.90
April	6000	4214.77	26400	5778.94
May	6000	3367.21	26400	4882.22
June	6000	3046.50	26400	3936.16
July	6000	2433.36	26400	2973.17
August	6000	2007.21	26400	2352.67
September	6000	2013.84	26400	4172.76
October	6000	2117.46	26400	2713.45
November	6000	2542.48	26400	5308.81
December	6000	4932.96	26400	16448.88
Annual	6000	3101.76	26400	5534.15

Note¹: As per ECA No. 5464-AKATW7, "Rated Capacity" is defined as the Annual Average Daily Flow for which the Sewage Treatment Plant is designed to handle. The Picton WWTP maintained an average daily flow within approved capacity requirements for the 2023 operational year.

Note²: As per ECA No. 5464-AKATW7, "Peak Flow Rate" is defined as the Peak Instantaneous Flow Rate, Peak Hourly Flow Rate or Peak Daily Flow Rate for which the Sewage Treatment Plant or treatment process unit or equipment is designed to handle, as appropriate. The Picton WWTP maintained Peak Daily Flow Rate within approved capacity requirements for the 2023 operational year.

Table 12: Historical Influent Flows, 2014-2023

Year	Total Influent Flow	Annual Average Daily Flow
	m ³	m ³ /day
2014	1146429.54	3140.90
2015	865255.04	2370.56
2016	977265.12	2670.12
2017	1308584.33	3585.16
2018	1142974.13	3131.44
2019	1195875.12	3276.37
2020	1023748.86	2793.59
2021	858004.49	2350.70
2022	1130953.94	3098.50
2023	1290722.78	3536.23

We note that for the previous year, and historically, the wastewater treatment plant appears to operate under both its rated and sustainable capacity.

The current number of sanitary service connections is not specifically known however the 2024 Master Servicing Plan suggests the plant serves 4,573 people. If you presume, on average, each connection has 3 people this would reflect 1,524 service connections.

Reserve Capacity Calculation

It is the position of the Province that the number of lots in approved plans of subdivision, developments committed by virtue of approved zoning, new official plans or site-specific official plan amendments, should not exceed the design capacity of the sewage and/or water system. To ensure that capacity is not exceeded it is necessary to determine what uncommitted reserve capacity is available.

To determine what capacity available at a municipal wastewater treatment plant the Ministry of Environment “Procedure D-5-1 Calculating and Reporting Uncommitted Reserve Capacity at Sewage and Water Treatment Plants” has been followed and summarized below:

$$C_u = C_r - (L \times F \times P)/H$$

C_u = Uncommitted Hydraulic Reserve Capacity (m^3/d)

C_r = Hydraulic Reserve Capacity (m^3/d)

L = Number of Unconnected Approved Lots

P = Existing Connected Population

H = Number of Households or Residential Connections

Wastewater Treatment Plant:

F = Average Day Flow per Capita ($m^3/capita/day$)

(Note: In this calculation average day flow is used for wastewater treatment plants while max day flow is used for water treatment plants.)

From the Picton WWTP 2023 Annual Report we know:

Plant Rated Capacity = 6,000 m^3/d

Sustainable Plant Capacity = 4,800 m^3/d

Average Daily Flow = 3,101 m^3/d^*

‘Sustainable’ Hydraulic Reserve Capacity (C_r) = 1,699 m^3/d

The number of unconnected approved lots includes:

1. Vacant lots/units in registered plans of subdivision and condominium.
2. Lots/units in draft approved plans of subdivision and condominium.
3. The maximum development potential of lands as permitted under existing zoning.
4. Registered plans of condominium.
5. Vacant lots created by consent in serviced areas.

For the purpose of this document the number of unconnected approved lots is assumed to be zero. This assumption is made to calculate the actual current hydraulic capacity available for additional development and acknowledges that only the Municipality will be aware of the status

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of all lands within the serviced boundary and therefore they are best equipped to determine if the calculated uncommitted reserve capacity is ultimately deemed acceptable.

Number of Unconnected Approved Lots (L) = 0

The existing Hydraulic Demand per lot is calculated by dividing Average Daily Flow 3,101 m³/d by the number of service connections 1,524 which equals 2.03 m³/d/connection.

If we consider the sustainable capacity, there is a Hydraulic Reserve Capacity of 1,699 m³/d. With a demand per connection of 2.03 m³/d there would be capacity at the wastewater treatment plant for approximately 837 additional connections.

The proposed development will include 98 residential connections and 1 potential future commercial connection.

It is understood that Picton, and Prince Edward County in general, is seeing a great deal of development activity. While there appears to be capacity for the proposed development, it is likely that the wastewater reserve capacity at the treatment plant will be exceeded in the very near future by developments that are, or will soon be, requesting capacity allocations.

Additionally, it is understood that the County is aware of this development and may already be considering the potential impacts on the wastewater distribution system. The image below is from a Public Consultation presentation from October 2024 and illustrates that the County is aware of the development. It should be noted that the number of proposed units has since been reduced from 164 to 98.

Development Name	Status	Total Units	Estimated Builds (Units)		Strong likelihood of immine		
			2025-2032	2033-2043	Imminent?	2025-2032	2033-2043
Talbot Ridge	Construction	25	25		Yes	25	0
West Meadows	Construction	550	550		Yes	550	0
Port Picton	Final Plan and Construction	200	200		Yes	200	0
Block 43 Condo	Site Plan Approved and Construction	8	8		Yes	8	0
Loyalist Heights	Second Submission	390	390		Yes	390	0
Barker Street/Anderson	Approved	500	400	100		0	0
Vineridge	Awaiting OLT Decision	560	300	260	Yes	300	260
Base 31	OPA Submission	7500	800	2000	Yes	800	2000
Talbot on the Trail	Construction	100	100		Yes	100	0
Cosmopolitan	Approved	164	164		Yes	164	0
Cork & Vine	Draft Plan Approved	2400	550	1850	Yes	550	1850
Tulip Subdivision	Third Submission	450	250	200		0	0
WBE PH1	Draft Plan Approved	176	150	26		0	0
80 Maple	Site Plan - needs Services	96	96		Yes	96	0
Former Duke Dome	Site Plan Approved	56	56		Yes	56	0
Sterling	Zoning Approved	253	253		Yes	253	0
Cold Creek	Technical Circulation	904	450	454	Yes	450	454
Fawcett	Statutory Public meeting	88	88		Yes	88	0
Wellings	Approved	65	65		Yes	65	0
Nicholas	Technical Circulation	104	104		Yes	104	0
Kaitlin (Willngtn West)	Technical Circulation	60	60			0	0
TOTAL		14649	5059	4890		4199	4564

2.2 WASTEWATER COLLECTION & CONVEYANCE

The County has previously completed a Municipal Class Environmental Assessment Report, Picton Sanitary Sewer and Watermain Extension Study (2006) and Addendum (2011).

The Picton Master Servicing Plan for Water, Wastewater, and Stormwater Management currently underway will be updating the County's plans as it relates to collection and conveyance plans. Until greater clarity on these plans is available, we will reference the 2011 plans.

When considering the sanitary servicing of the subject lands the following adjacent existing infrastructure is referenced in these earlier studies and relevant to this development:

1. Gravity sewer main ending just north of Finnegan Drive.

As we do not have access to the County's sanitary network modelling, existing flows across the network are not known and comment cannot be provided. We understand that the sanitary network modelling does include provisions for this development.

2.3 WASTEWATER DEMAND AND PIPE CAPACITY

Greer Galloway has completed a preliminary design for the sanitary servicing for this development and an associated sanitary sewer design sheet, which is appended to this report. It should be noted that the design sheet only includes information for the pipe run adjacent to the municipal connection for the purpose of demonstrating that a 200 mm sanitary sewer at 0.5% has more than sufficient capacity to support this development. The following has been considered in this design:

- Total number of residential units: 98
- 2008 Ministry of Environment Design Guidelines for Sewage Works recommends average daily domestic demands in the range of 225 – 450 l/capita/day. We understand that the County utilizes a design flow of 320 l/capita/day.
- Being a new development with new fixtures, etc. it is likely that the demands will fall in the low end of this range.
- For the purpose of this estimate, the County's standard of 320 liters per day has been utilized.
- Assuming average occupancy of 3 people per residential unit, a total population of 294 people results – this is felt conservative as some of the housing styles are more likely to have fewer than 3 occupants but will be assumed for the purpose of this document.
- At 320 l/capita/day an average daily demand of 94,080 l/d (1.09 l/s) can be expected.

- A peaking factor of 2.0 for maximum day calculations and 3.0 for peak hour calculations have been utilized as per MECP Table 3.1 and conjunction with the County. This results in a maximum daily demand of 2.18 l/s and a peak hour demand of 3.27 l/s.
- As per MOE design guidelines, an allowance for extraneous demands, resulting system leakage, unintended connections, etc. of an additional 0.90 l/s (0.28L/s/ha) is allowed for.
- Total peak maximum daily demand in the order of 3.08 l/s and total peak hourly demand of 4.17 l/s have been calculated.
- This flow is roughly equivalent to the minimum sanitary gravity sewer size of 200mm installed at its minimum slope of 0.5% flowing 23% full; or a 300mm gravity sanitary sewer installed at its minimum slope of 0.5% flowing 8% full. It should be noted that there will be some potential additional flow from the future commercial development or adjacent residences that may elect to connect in the future. These flows are anticipated to be minimal and will not substantially affect the proposed sanitary infrastructure.

As previously noted, the servicing for this development is dependent on the construction of a future sanitary main on County Road 49 (Picton Main Street). To facilitate the use of gravity sewers at this development, two service connections will be required. These service connections will be located at each of the proposed entrances. The elevation of the County sewer will need to be a maximum of 90.48 m at the north entrance and 88.90 m at the south entrance to ensure the proper operation of the onsite gravity sewers. It is understood that this has been discussed with the County and that the necessary provisions have been made to accommodate this requirement. The proposed development will tie-into these existing service stub elevations at the property line.

2.4 INTERNAL PIPE NETWORK

As per the appended preliminary drawings, a draft design has been completed for the internal sanitary main at the subject development. A series of gravity sewer mains will be constructed following the internal road network within the development. These sewers will provide service to all 18 buildings within the development and will connect to the future sanitary main on County Road 49. The proposed design includes the following:

- A series of 200mm PVC gravity sanitary sewer mains. These sewers have been designed at between 0.5-2.0%. All slopes in this range provide sufficient capacity and meet MOE requirements for minimum and maximum pipe velocity.
- We understand that two service connections (one at each entrance) are being provided for this development. However, for the purpose of the draft design, only the south connection has been utilized. It is possible that the sanitary stub located at the north entrance to the development could be utilized, if required.
- 98 individual 100 mm diameter sanitary connections to the proposed buildings.
- 1200 mm manhole junction structures at all changes in horizontal sewer alignment or at maximum intervals as prescribed in the MOE design guidelines. Fall across manhole

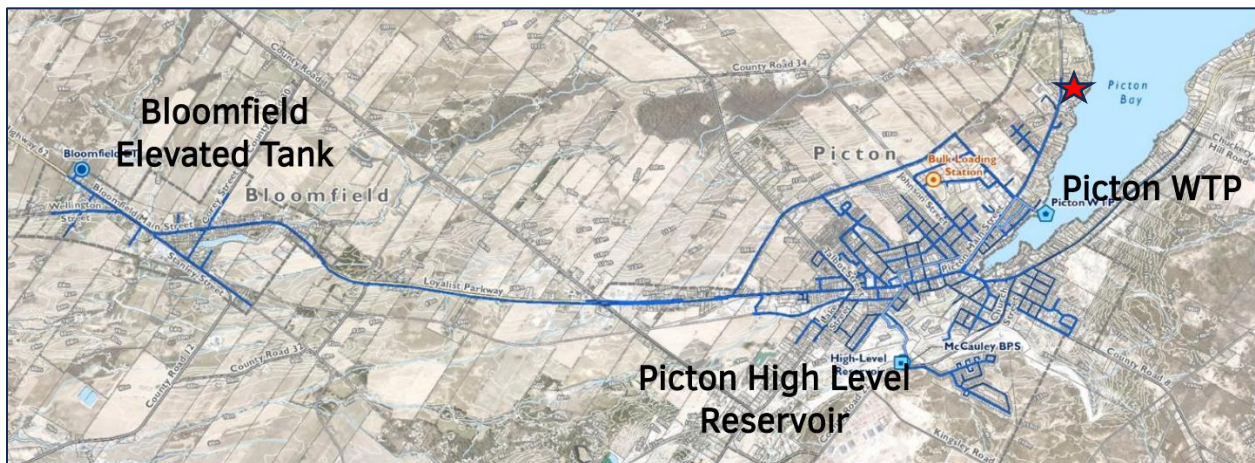
structures has been provided as per the above noted guidelines. A sanitary manhole has also been provided at the limits of the County ROW.

All collection works will be new construction and sized as required consistent with current standards and good practices under Ministry of Environment, Conservation and Parks Environmental Compliance Approval.

3 WATER SUPPLY & TREATMENT

3.1 PICTON WATER TREATMENT PLANT

The Picton Water Treatment Plant is located at 30 Spencer Street and serves both the Town of Picton as well as the Village of Bloomfield approximately 7 km to the west.



The Cosmopolitan Homes development lands are shown by the star in the image above. The existing water distribution system extends to the subject property, and is scheduled to be upgraded as part of an upcoming infrastructure project.

Flows are conveyed from the water treatment plant location on the shore of Picton Bay at the end of Spencer Street. Water storage is provided by an at grade reservoir located on the escarpment on County Road 22.

As part of the development of the subject lands, an expansion to the existing distribution network will be required in addition to additional treatment demand at the water treatment plant.

Similar to the earlier wastewater comments, Prince Edward County has undertaken the Picton Master Servicing Plan for Water, Wastewater, and Stormwater Management.

Picton Master Servicing Plan – Drinking Water Findings (October 2nd, 2024, Public Meeting)

- Constructed in 1928, upgraded in 2009.

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- Serves 2,996 people.
- Rated capacity of the plant is 10,400 m³/d.
- Stress test capacity – 6,000 m³/d.
- Sustainable plant capacity based on staff experience = 5,200 m³/d.

2023 Picton Water Treatment Plant & Water Distribution System Annual Report

1. Drinking Water System Number 220000987
2. Max Daily Capacity: 10,400 m³/day
3. Flow records are provided in the excerpt from the report below:

Picton DWS: Treated Water Flows 2023				
Month	Total Flow	Minimum Daily Flow	Average Daily Flow	Maximum Daily Flow
	m³	m³	m³	m³
January	65279.50	1103.19	2105.79	3128.25
February	55053.71	1540.25	1966.20	2503.90
March	57144.74	1313.38	1843.38	2581.77
April	60396.31	1556.05	2013.21	3440.84
May	74468.93	1733.14	2402.22	3100.09
June	82282.84	1947.67	2742.76	5256.26
July	85567.50	2065.08	2760.24	3251.84
August	81733.88	1825.55	2636.58	3178.62
September	83006.37	2237.79	2766.88	3398.47
October	73130.73	1743.95	2359.06	2931.39
November	64007.10	1694.07	2133.57	2596.93
December	65092.37	1458.28	2099.75	2691.85
Annual Total	847163.97	1103.19	2319.14	5256.26

Picton DWS: Treated Water Flow Comparison		
Rated Capacity as per MDWL/DWWP	10400 m ³	% of Maximum
Actual Maximum Daily Capacity	5256.26 m ³	50.54 %
Actual Mean Daily Capacity	2319.14 m ³	22.29 %

Reserve Capacity Calculation

The process for determining the reserve capacity of the drinking water plant is like that of the wastewater treatment plant and follows MOE Procedure D-5-1. The calculation is summarized below:

$$C_u = C_r - (L \times F \times P)/H$$

C_u = Uncommitted Hydraulic Reserve Capacity (m³/d)

C_r = Hydraulic Reserve Capacity (m³/d)

L = Number of Unconnected Approved Lots

P = Existing Connected Population

H = Number of Households or Residential Connections

Water Treatment Plant:

F = Maximum Daily Flow per Capita ($\text{m}^3/\text{capita}/\text{day}$)

(Note: In this calculation max day flow is used for water treatment plants while average day flow is used for wastewater treatment plants.)

As noted previously, we know:

Plant Rated Capacity = $10,400 \text{ m}^3/\text{d}$

Street Test Capacity = $6,000 \text{ m}^3/\text{d}$

Sustainable Plant Capacity = $5,200 \text{ m}^3/\text{d}$

2023 Maximum Daily Flow = $5,256 \text{ m}^3/\text{d}$

* It appears the sustainable plant capacity has already been exceeded when the sustainable capacity based on staff experience is considered.

Similar to the wastewater treatment plant, it appears likely that the imminent development demands will exceed plant capacity in the near future, if it hasn't already. This will be confirmed as part of the Picton Master Servicing Plan currently underway.

3.2 WATER DISTRIBUTION SYSTEM

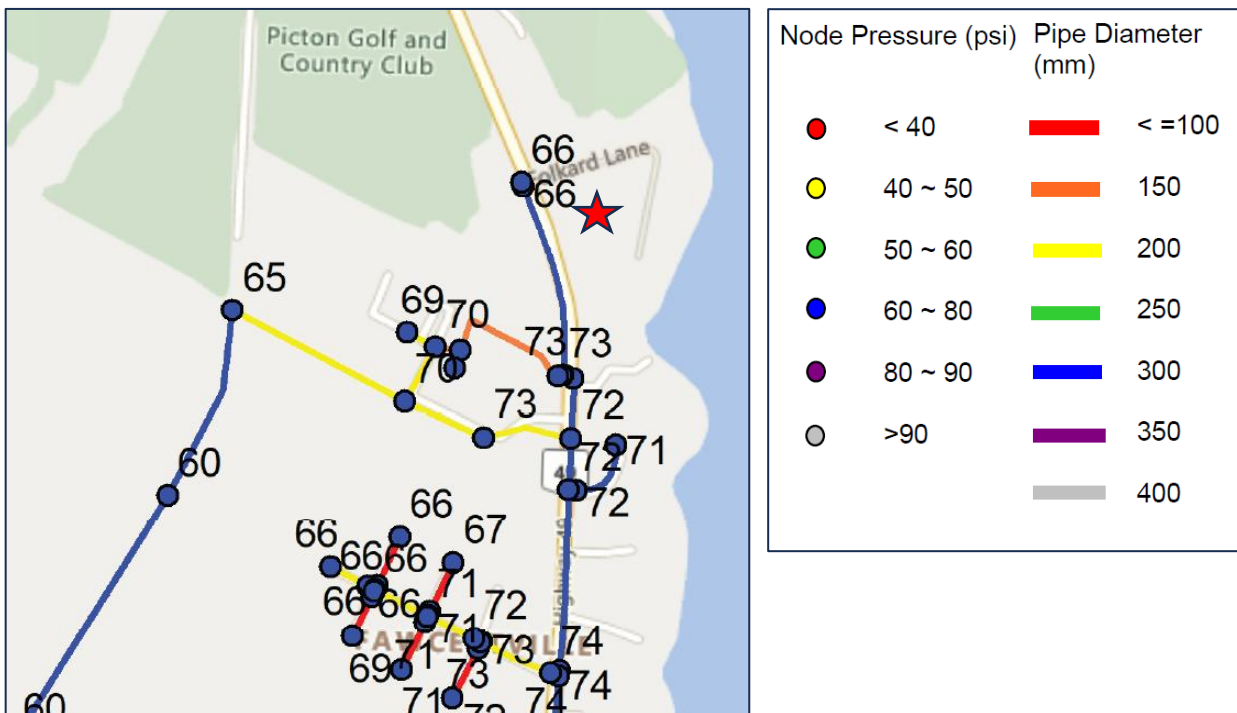
We understand the Municipality maintains a computer model of the Picton / Bloomfield water distribution system. It is our understanding that the municipal model includes provisions for this development, and that this includes a development of 166 units and population of 399, both of which exceed the updated design for Folkard Lane. Additionally, the model assumes an average day demand of 1.48 l/s, a maximum day demand of 2.95 l/s and a peak hour demand of 4.43 l/s. It is noted that the maximum day demand is slightly less than has been calculated in Section 2.2 of this report, and it is likely that this is due to a less conservative extraneous flow calculation in the County's model. It is recommended that this is reviewed in conjunction with this report.

The following can be provided based on the records and information provided by the County:

Water Network Conditions – December 2022

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During ADD (Assumed to mean Average Day Demands)

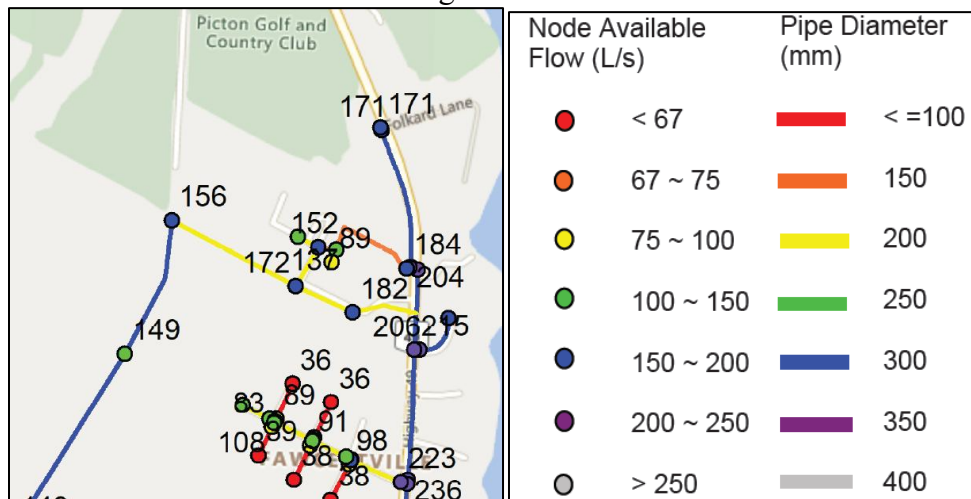


The approximate location of the subject lands are denoted by the star in the above image.

From these images, the following general observations can be made in the area of the development:

- Nodes immediately adjacent to the development indicate pressure at 66 psi.
- Nodes further south along Main Street see pressures increase to the 70-75 psi range.
- Existing pipe size on Picton Main Street is 200 mm but will be upgraded to 300 mm as part of an upcoming construction project.

Data on available Fire Flow during MDD is shown below:



From these images, the following general observations can be made in the area of the development:

- Nodes immediately adjacent to the proposed development suggest fire flow capacity of 171 l/s (2710 USgpm).

Regarding anticipated static pressure for the proposed development, it is noted that the pressure is largely a function of elevation and with the proposed development being similar in elevation to the immediately adjacent points, it is likely that pressures similar to or higher than these will be found. MOE standards recommend normal operating pressures in the 50 psi to 70 psi range with a minimum of 40 psi. The available information suggests operating pressures for the development will meet these requirements.

It is noted that 3-storey townhouses are proposed as part of the development. It is recommended that the municipal model be reviewed to ensure that the height of the proposed units does not result in an unacceptable reduction in pressure at the upper floors.

Regarding fire flow for the proposed development, preliminary calculations have been completed based on the Fire Underwriters Survey (FUS) requirements. We understand the County references the FUS when assessing development approval processes.

For reference, calculations have been completed for a representative building in this development. The following assumptions have been made with respect to the proposed building in this calculation:

- Wood Frame Type 5
- Storeys: 3
- Largest townhouse building – 771 m² (10 units)
- No fire separations between units.
- Limited combustible interior.
- No sprinkler.

The FUS states that the required fire flow is calculated on the following basis:

$$RFF = 220C\sqrt{A}$$

RFF = the Required Fire Flow in litres per minute (LPM).

C = the Construction Coefficient related to the type of construction of the building.

A = the Total Effective Floor Area (effective building area) in square meters of the building.

The C value for Wood Frame Construction Type 5 is 1.5.

The Total Effective Floor Area is 771 m².

Therefore, the unadjusted RPF is 9,163 LPM or 153 L/s.

The County maintains a computer model of their water distribution network that we understand contains provisions for this development. We recommend the subject development be updated in the model based on the recommendations of this report to confirm the ability to provide fire flow for the subject development.

3.3 PRELIMINARY WATER DISTRIBUTION SYSTEM DESIGN

A preliminary layout of the water distribution system for this development has been shown on the appended servicing drawings. This system includes the following parameters:

- Connection to 200 mm watermain stubs at both entrances. It is our understanding that these stubs are to be installed as part of the ongoing Main Street Reconstruction project.
- A 200 mm watermain network throughout the proposed development. The adequacy of this network should be confirmed in conjunction with the municipal model based on the parameters shown on the preliminary design drawings. The watermain has been designed with appropriate separation from adjacent storm/sanitary as per MECP standards, and adheres to requirements for frost or provides insulation where appropriate.
- Individual 25 mm water services to each unit. It should be noted that all services will come complete with a curb stop.
- Water valves as per MECP requirements.
- Precast concrete chamber for metering and backflow prevention at the limits of the County ROW.
- Hydrants placed as per Ontario Building Code requirements.

4 CONCLUSIONS

The following summarizes the findings of this report as it relates to the proposed Cosmopolitan Homes Development:

Sanitary Servicing

1. The Picton Master Servicing Plan suggests the wastewater treatment plant's actual capacity is significantly less than the rated capacity and will not provide sufficient capacity for many of the planned developments. Until this master plan is completed, the ability to service the subject lands and other developments will remain unclear. It should be noted that the proposed development has been included in planning documents published by the County in consideration of available servicing capacity.
2. The subject lands can be adequately serviced by gravity sewers if the proposed sanitary sewer on Main Street is constructed at the elevations noted in this report.
3. A sanitary main following the service roads on the property will connect to individual services for each unit. It is our understanding that two connections to the future sanitary main on County Road 49 will be provided to convey flows to the municipal system. However, our preliminary servicing design only utilizes one of these connections.

September 21st, 2026

4. A preliminary servicing design for the proposed sanitary network has been appended to this report.

Water Servicing

1. The Picton Master Servicing Plan suggests the water treatment plant's actual capacity is significantly less than the rated capacity and will not provide sufficient capacity for many of the planned developments. Until this master plan is completed, the ability to service the subject lands and other developments will remain unclear. It should be noted that the proposed development has been included in planning documents published by the County in consideration of available servicing capacity.
2. There is an existing 200 mm watermain that fronts the proposed development. Our understanding is that this watermain is anticipated to be replaced as part of an upcoming infrastructure project with a 300 mm watermain.
3. Pressures ranging from 50 psi to 70 psi with a minimum of 40 psi are required by the MOE. Based on existing records, it appears these pressure requirements can be met by the proposed development, but this should be confirmed by the municipal water network model.
4. Based on existing records, it appears fire flows in the range of 2710 USgpm are possible in the development, but this should be confirmed by the municipal water network model. While example fire flow calculations are provided in this report, results must be confirmed based on actual building construction design. Should any concerns exist as it relates to the ability of the municipal network to supply the required fire flow, further consultation with the County will be required.
5. A preliminary servicing design for the proposed water network has been completed and is appended to this report. It is recommended that the stated conditions and elevations be reviewed in conjunction with the model for the County's watermain network.

This report is provided in support of the proposed development on Folkard Lane. Understanding this is the initial step in the development application and approval process changes in the development plans can be expected. The statements made in this report are of a general nature and some variation in development conditions can occur without requiring revision or amendment.

Respectfully Submitted,



Rob Asselstine, C.Tech
Senior Civil Technologist



Kevin Hawley, P.Eng
Operations Manager

Appendix A

Preliminary Servicing Drawings

FILE NAME: I:\C:\G\J\25035A\25035A - 250317.DWG - 250317.DWG - FOLKARD LANE DEVELOPMENT (25-035A) - 250317.DWG - LAYOUT COVER SHEET (25-035A) - 250317.DWG



PROJECT LOCATION

FOLKARD LANE DEVELOPMENT

FOLKARD LANE, PICTON, K0K 2T0
PRINCE EDWARD COUNTY, ON

CIVIL - LIST OF DRAWINGS

C1	SERVICING PLAN
C2	GRADING PLAN
C3	PROPOSED PLAN & PROFILE - FOLKARD LN
C4	PROPOSED PLAN & PROFILE - INTERIOR STREETS
C5	PRE DEVELOPMENT CATCHMENT AREAS
C6	POST DEVELOPMENT CATCHMENT AREAS
C7	POST DEVELOPMENT OVERALL AREA
D1	DETAILS

CLIENT LOGO

3	2026-09-22	AH	ISSUED FOR DRAFT PLAN OF SUB-DIVISION
2	2026-01-16	SH	ISSUED FOR STORMWATER REPORT
1	2026-01-09	SH	ISSUED FOR 80% REVIEW
No.	YYYY-MM-DD	BY	DESCRIPTION



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jp2g PROJECT No.: 25-5035A

SHEET #

COVER



CLIENT LOGO

KEYPLAN (N)

DISCLAIMER NOTES

LEGEND

3	2026-09-22	AH	ISSUED FOR SERVICING REPORT
2	2026-01-16	SH	ISSUED FOR STORMWATER REPORT
1	2026-01-09	SH	ISSUED FOR 80% REVIEW

STAMP

STAMP

LICENSED PROFESSIONAL ENGINEER
K. T. HAWLEY
100149851
2026-09-22
PROVINCE OF ONTARIO

PROJECT

FOLKARD LANE DEVELOPMENT

FOLKARD LANE, PICTON, K0K 2T0
PRINCE EDWARD COUNTY, ON

DRAWING

SERVICING PLAN

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JP2g PROJECT No.: 25-5035A

NORTH

TRUE NORTH

CLIENT No.:

DRAFTED: A. HICKS

DESIGNED: R. ASSELSTINE

REVIEWED: K. HAWLEY

APPROVED: K. HAWLEY

SHEET#

C1



CLIENT LOGO

KEYPLAN (N)

DISCLAIMER NOTES

3

2026-09-22

AH

ISSUED FOR SERVICING REPORT

2

2026-01-16

SH

ISSUED FOR STORMWATER REPORT

1

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No. YYYY-MM-DD

BY

DESCRIPTION

STAMP

LICENSED PROFESSIONAL ENGINEER

K. T. HAWLEY

100149851

2026-09-22

PROVINCE OF ONTARIO

PROJECT

FOLKARD LANE DEVELOPMENT

FOLKARD LANE, PICTON, K0K 2T0
PRINCE EDWARD COUNTY, ON

DRAWING

PLAN & PROFILE
FOLKARD LANE

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NORTH

AS SHOWN
IN PLAN VIEW

CLIENT No.:

DRAFTED: A. HICKS

DESIGNED: R. ASSELSTINE

REVIEWED: K. HAWLEY

APPROVED: K. HAWLEY

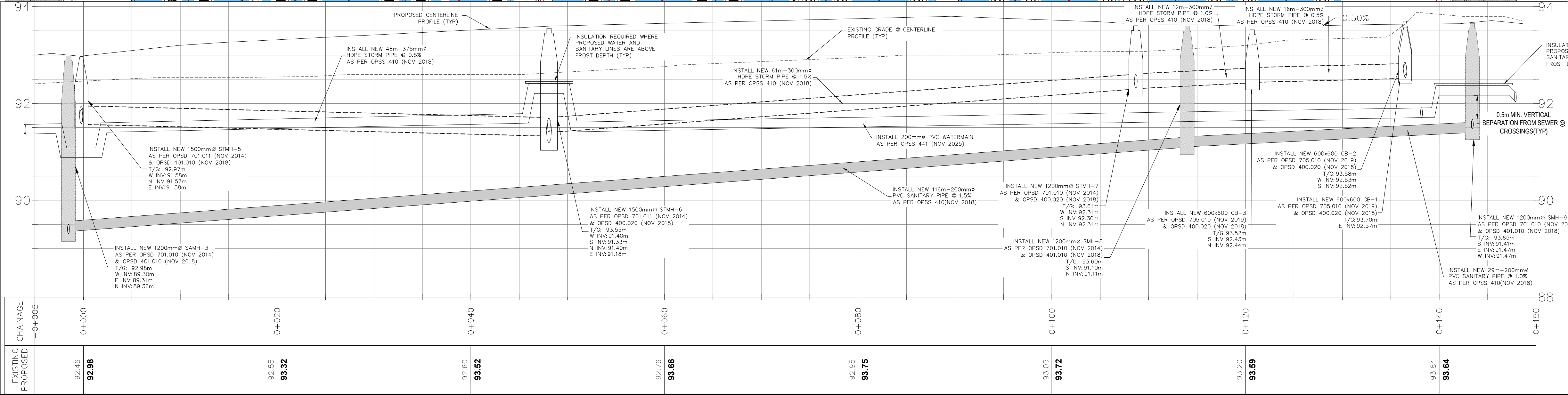
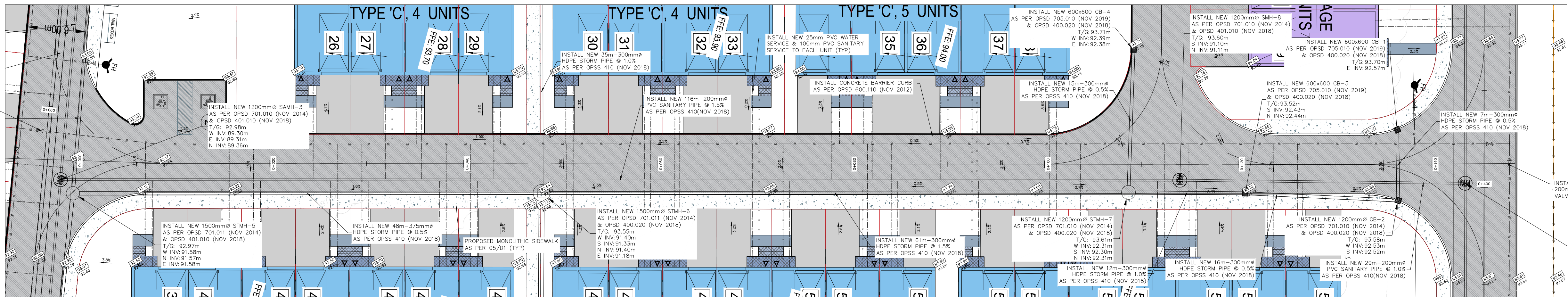
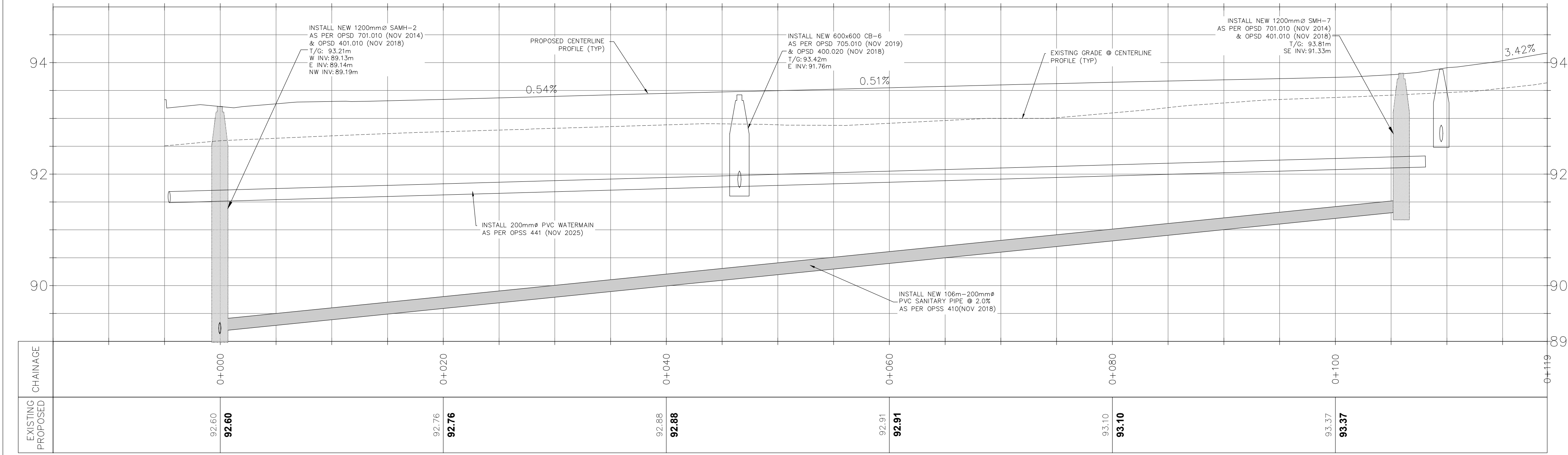
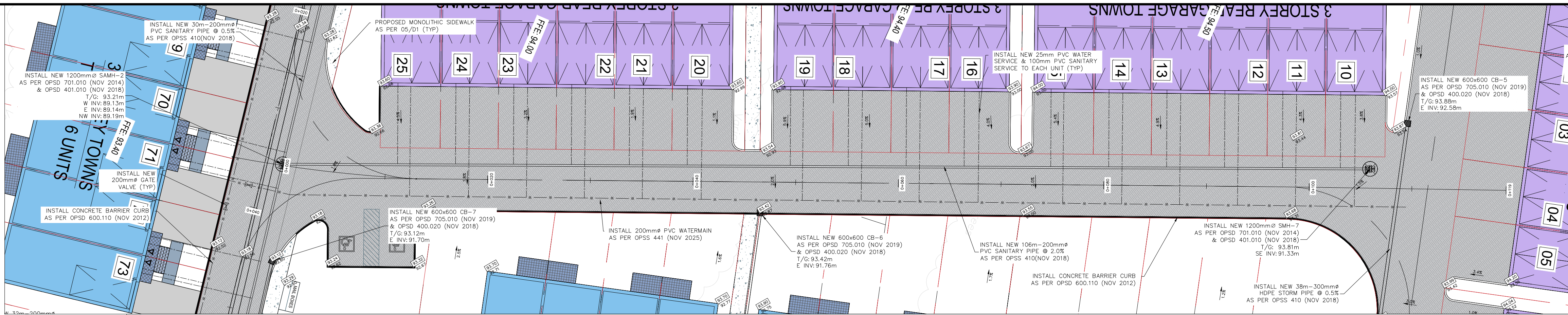
SCALE

SCALE AS SHOWN

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V= 1:70

SHEET#

C3



CLIENT LOGO

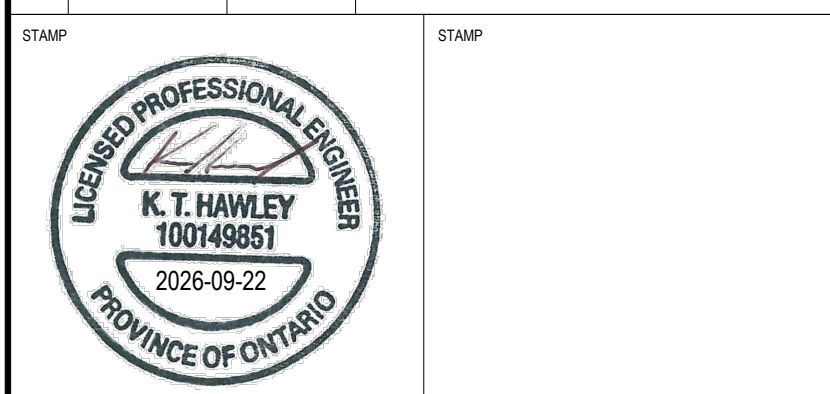
KEYPLAN (N/S)



DISCLAIMER NOTES

1. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ISSUES/PROBLEMS WHICH MAY OCCUR AS A RESULT OF A FAILURE TO FOLLOW THESE PLANS, SPECIFICATIONS AND THE DESIGN INTENT THEY CONVEY.
2. WHERE THERE ARE ALLEGED ERRORS, OMISSIONS, INCONSISTENCIES OR AMBIGUITIES PRESENT IN THE CONTRACT DOCUMENTS, THE CONTRACTOR MUST SEEK CLARIFICATION FROM JP2G. ANY COSTS OR SCHEDULE DELAYS WHICH RESULT AS A FAILURE TO CONTACT JP2G FOR DIRECTION SHALL BE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.
3. DO NOT SCALE DRAWINGS. REFER ANY DIMENSIONAL CLARIFICATIONS AND/OR POSSIBLE TRADE INTERFERENCES/CONFLICTS TO JP2G FOR CLARIFICATION PRIOR TO COMMENCEMENT OF THE WORK.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL COORDINATION WITH SUBTRADES AND SHALL ADDRESS CONSTRUCTION TEAM COORDINATION ITEMS PRIOR TO ISSUING REQUESTS FOR INFORMATION FROM JP2G.
5. THE POSITION OF POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWING, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, THE CONTRACTOR SHALL INFORM THEMSELVES OF THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND SHALL ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

No.	YYYY-MM-DD	BY	DESCRIPTION
3	2026-09-22	AH	ISSUED FOR SERVICING REPORT
2	2026-01-01	SH	ISSUED FOR STORMWATER REPORT
1	2026-01-09	SH	ISSUED FOR 80% REVIEW



PROJECT
FOLKARD LANE DEVELOPMENT
FOLKARD LANE, PICTON, K0K 2T0
PRINCE EDWARD COUNTY, ON

DRAWING
**PLAN & PROFILE
INNER STREETS**

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JP2g PROJECT No.: 25-5035A

NORTH

CLIENT No.:
DRAFTED: A. HICKS
DESIGNED: R. ASSELSTINE
REVIEWED: K. HAWLEY
APPROVED: K. HAWLEY

SCALE
0m 4 1250 H=1250 V=150
0 2 4 6 8 10m

SHEET#
C4

1. ALL WORKS TO BE COMPLETED IN ACCORDANCE WITH CURRENT ONTARIO BUILDING CODE, ONTARIO PROVINCIAL STANDARD SPECIFICATIONS AND DRAWINGS UNLESS SPECIFIED OTHERWISE.
2. PRIOR TO CONSTRUCTION, CONTRACTOR TO VERIFY ALL DIMENSIONS AND UTILITY LOCATES AND IDENTIFY POSSIBLE CONFLICTS.
3. ALL ENVIRONMENTAL PROTECTION MEASURES ARE THE RESPONSIBILITY OF THE CONTRACTOR.
4. ANY TRAFFIC SIGNS OR INFORMATION SIGNS REMOVED/DAMAGED DURING CONSTRUCTION ARE TO BE REPLACED AT THE CONTRACTORS EXPENSE.
5. ALL UTILITY POLES TO BE BRACED AS NECESSARY.
6. THE LOCATIONS OF UTILITIES ARE APPROXIMATE ONLY, AND THE EXACT LOCATION SHOULD BE DETERMINED BY CONSULTING THE MUNICIPAL AUTHORITIES AND UTILITY COMPANIES CONCERNED. THE CONTRACTOR SHALL PROVE THE LOCATION OF UTILITIES AND SHALL BE RESPONSIBLE FOR ADEQUATE PROTECTION FROM DAMAGE DURING CONSTRUCTION.
7. ALL WORK SHALL BE IN ACCORDANCE WITH RELEVANT CODES AND GUIDELINES.
8. ALL DRAWINGS AND ADDENDA ARE TO BE READ AS, AND IN CONJUNCTION WITH THE SPECIFICATIONS.
9. ALL MATERIALS SHALL BE INSTALLED AS SPECIFIED OR USING AN APPROVED EQUIVALENT.
10. CONTRACTOR IS RESPONSIBLE FOR VERIFICATION OF ALL DIMENSIONS BEFORE PROCEEDING WITH WORK.
11. ANY CHANGES MUST BE APPROVED BY THE ENGINEER.
12. ALL EXCESS MATERIAL TO BE REMOVED FROM THE MAIN TO PROPERTY LINE.
13. IF DEEMED SUITABLE BY CONTRACT ADMINISTRATION; EXCAVATED MATERIAL MAY BE USED TO COMPLETE TRENCH BACKFILLING, EXCEPT THAT NO BOULDERS SHALL BE ALLOWED. ALL COMPACTION OF TRENCH TO BE A MINIMUM OF 95% STANDARD PROCTOR DENSITY.
14. SIDEWALKS AND CURBS ARE TO BE DEPRESSED AT ALL EXISTING ENTRANCES. MAXIMUM SLOPE OF 1:20 AS PER 80.23 ACCESSIBILITY FOR ONTARIANS WITH DISABILITIES ACT (2005).
15. HOPE CULVERTS ONLY TO BE REQUIRED IN SITUATIONS WHERE COVER EXCEEDS 300 MM. IN LOW COVER APPLICATIONS WHERE CSP MUST BE USED, A MINIMUM OF 150 MM GRANULAR 'A' BELOW ASPHALT SHALL BE INSTALLED AND A FROST TAPER BE USED AS PER OPSD 803.031 (NOV 2015).

1. ALL TRAFFIC CONTROL/PEDESTRIAN SIGNING AS PER OTM. 80 O/C 7.

1. FOR AREAS OF EARTH SUBGRADE, GRANULAR DEPTHS ARE TO BE 150 MM GRANULAR A AND 300MM GRANULAR B TYPE 2. PROVIDE 3% CROSSFALL ON SUBGRADE.
2. CONCRETE CURB AS PER OPSPD 600.040 (NOV 2012).
3. CONCRETE SHALL BE CLASS C2, 30 MPA 28 DAY STRENGTH, 19MM COARSE AGGREGATE NOMINAL MAXIMUM SIZE, 6.0% TO 8.0% AIR CONTENT.
4. CONCRETE MATERIALS AND PRODUCTION AS PER OPSS 350 (MAR 1998), 351 (NOV 2015), 353 (NOV 2016), 1301 (NOV 2007), 1302 (SEPT 2016), 1350 (NOV 2016).
5. BOULEVARDS AND LANDSCAPE AREAS TO BE FINISHED WITH 50MM OF TOPSOIL AND SEED, OR TO PRE-EXISTING CONDITION.
6. COMPACTION OF THE SUBGRADE FOR ASPHALT SHALL BE AS PER OPSS 501 (NOV 2017). COMPACTION TESTING MAY BE CARRIED OUT AT THE DISCRETION OF THE CONTRACT ADMINISTRATOR.
7. ANY GRANULAR MATERIAL INCORPORATED INTO THE WORK SHALL BE AS PER OPSS 1010 (NOV 2013) AND OPSS 314 (NOV 2016).
8. DEPTH OF GRANULAR "A" UNDER NEW SIDEWALK SHALL BE 150 MM.
9. CONTRACTOR IS RESPONSIBLE FOR THE SHORING OF ALL EXCAVATIONS IF REQUIRED TO PROTECT ADJACENT INFRASTRUCTURE.
10. IN AREAS WHERE COMPACTION CANNOT BE ACHIEVED, THE CONTRACTOR SHALL USE NON-SHRINK FILL AS THE BACKFILL MATERIAL AS PER OPSS 1359 (NOV 2016).

1. FINISHED SURFACES SHALL BE AT A MINIMUM GRADE OF 2% UNLESS OTHERWISE NOTED.
2. SLOPES IN LANDSCAPED AREAS SHALL NOT EXCEED 3:1.
3. ALL EXISTING ELEVATIONS AND GRADES ARE TO BE VERIFIED BY THE CONTRACTOR PRIOR TO GRADING.
4. ALL GRADED SURFACES SHALL BE GRADED TO PREVENT POONDING AND WITHOUT LOW AREAS EXCEPT WHERE APPROVED SWALE OR CATCHBASIN OUTLETS ARE PROVIDED.
5. THE CONTRACTOR IS RESPONSIBLE FOR REVIEWING PROPOSED GRADES WITH CONFLICTS REGARDING THE PROPOSED STRUCTURES.
6. DRAINAGE DITCHES SHALL BE GRADED AT A MINIMUM OF 2%, UNTIL A LOWER DITCH IS ENCOUNTERED OR THE EXISTING SURFACE DRAINS AWAY FROM THE SLOPED SUB-AREA.
7. CONTRACTOR TO RESTORE ALL DISTURBED AREAS WITH 50MM OF TOPSOIL AS PER OPSS 804 OR TO PRE-EXISTING CONDITIONS.

1. DRIVEWAY REMOVALS- LIMITS TO BE MARKED IN FIELD WITH CONTRACT ADMINISTRATOR AT PROPERTY LINE OR AS MARKED
2. CONTRACTOR IS RESPONSIBLE FOR DISPOSAL OF EXCESS MATERIAL AS PER OPSS 510

1. ALL EROSION AND SEDIMENT CONTROLS SHALL BE INSTALLED PRIOR TO CONSTRUCTION AND MONITORED AND MAINTAINED BY THE CONTRACTOR THROUGHOUT THE CONSTRUCTION PROCESS, UNTIL ALL DISTURBED AREAS HAVE BEEN RE-VEGETATED. THE TEMPORARY SEDIMENT AND EROSION CONTROL MEASURES MUST BE REMOVED ONCE THE SITE HAS BEEN STABILIZED/COMPLETED AND SITE WORK IS COMPLETE.
2. ALL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE INSPECTED AFTER EACH RAINFALL TO THE SATISFACTION OF THE CONTRACT ADMINISTRATOR.
3. ANY DISTURBED AREAS NOT SCHEDULED FOR FURTHER CONSTRUCTION WITHIN FORTY-FIVE (45) DAYS WILL BE PROVIDED WITH A SUITABLE TEMPORARY MULCH AND SEED COVER WITHIN SEVEN (7) DAYS OF COMPLETION.
4. REGARDLESS OF SITE SPECIFIC ITEMS DETAILED ON THE PLANS, THE CONTRACTOR SHALL INSTALL EROSION CONTROL MEASURES TO SUIT THE PROPOSED WORK METHODS TO CONTROL SEDIMENT FROM RUNNING OFF THE SITE PRIOR TO ANY DISTURBANCE.
5. ALL DISTURBED AREAS TO BE REVEGETATED TO ORIGINAL CONDITION OF BETTER UNLESS OTHERWISE SPECIFIED. REVEGETATION OF DISTURBED AREAS IS TO BE COMPLETED AS SOON AS POSSIBLE. RESTORATION SHALL PROVIDE A MINIMUM OF (1) TOPSOIL AND SEED/SOD.
6. ALL ROADS USED TO ACCESS THE SITE SHALL BE KEPT CLEAN TO THE SATISFACTION OF THE CONTRACT ADMINISTRATOR.

1. WHILE UNDERTAKING CLEARING, DEMOLITION, EXCAVATION OR CONSTRUCTION THE CONTRACTOR SHALL BE VIGILANT FOR THE POTENTIAL PRESENCE OF UNDERGROUND OIL TANKS, CONTAMINATED SOIL OR GROUNDWATER, BURIED WASTES OR ABANDONED WATER WELLS. IF ANY OF THE ABOVE ARE ENCOUNTERED OR SUSPECTED, THE OWNER SHALL ENSURE THAT: ANY WASTES GENERATED BY SITE CLEAN-UPS ARE MANAGED IN ACCORDANCE WITH APPLICABLE LAWS AND STANDARDS;
2. CONSTRUCTION WASTES ARE NOT TO BE BURIED WITHIN THE PROPERTY THAT IS THE SUBJECT OF THIS AGREEMENT, AND THAT THE OWNER AND THEIR CONTRACTORS REPORT ALL SPILLS TO THE MINISTRY OF THE ENVIRONMENT'S SPILLS ACTION CENTRE (1-800-268-6060) AND TO PRINCE EDWARD COUNTY (Y) FORTHWITH.

1. WHERE IN EARTH SUBGRADE GRANULAR DEPTHS TO BE 150 MM GRANULAR A AND 400MM GRANULAR B TYPE 2. PROVIDE 3% CROSSFALL ON SUBGRADE.
2. CONCRETE SHALL BE CLASS C2, 32 MPA 28 DAY STRENGTH, 19MM COARSE AGGREGATE NOMINAL MAXIMUM SIZE, 6.0% TO 8.0% AIR CONTENT.
3. CONCRETE MATERIALS AND PRODUCTION AS PER OPSS 350, 351, 352, 353, 1301, 1302 AND 1350.
4. CONCRETE SHALL BE FINISHED WITH 100MM OF TOPSOIL AND SEED, OR TO PRE-EXISTING CONDITION. THE SWALE WILL BE SODDED AS PER OPSS 803
5. SIDEWALKS AND CURBS ARE TO BE DEPRESSED AT ALL EXISTING ENTRANCES. MAXIMUM SLOPE OF 1:20 AS PER 80.23-ACCESSIBILITY FOR ONTARIAN'S WITH DISABILITIES ACT (2005).

1. PARKING LOT ASPHALT SHALL BE 60 MM HL3 SURFACE COURSE.
2. ASPHALT SHALL BE AS PER OPSS 310 (NOV 2017) AND OPSS 1150 (NOV 2010). THE PGAC SHALL BE 58-28
3. A TOTAL OF 3 BULK ASPHALT SAMPLES SHALL BE TAKEN AS PER OPSS 310 (NOV 2017).

1. SWALES TO BE SODDED AS PER OPSS 803 (APR 2018).
2. RODENT GATES TO BE INSTALLED AT ALL PIPE OUTLETS AS PER OPSS 405.

1. HYDRANTS SHALL BE MUELLER CENTURY OR APPROVED EQUIVALENT ANSI STANDARD C502-94.
2. WATERMAINS SHALL BE PVC CLASS 150 AND CONFORM TO AWWA STANDARD C 900-94 OR DUCTILE IRON, CEMENT MORTAR LINED TO ANSI STANDARD C151/41-51-02.
3. TRACER WIRE SHALL BE INSULATED COPPER WIRE TO BE INSTALLED ON ALL PVC WATERMAINS AND PE SERVICE CONNECTIONS.
4. TRACER WIRE SHALL BE GROUNDED TO EACH FITTING.
5. GATE VALVE SHALL BE MUELLER SUPER-SEAL RESILIENT SEAR OR APPROVED EQUIVALENT. CURB STOPS SHALL BE LOCATED 0.3m OFF THE PROPERTY LINE.
6. EXISTING WATER MAINS SHALL BE DIRECT TAPPED BY MUNICIPAL FORCES.
7. PEO WATER AND WASTEWATER STAFF SHALL BE ONSITE FOR ALL TIE-INS AND CONNECTIONS TO BOTH THE WATER AND WASTEWATER SYSTEMS.



219.100 LIGHT DUTY STRAW BALE BARRIER
219.110 LIGHT-DUTY SILT FENCE BARRIER
310.010 CONCRETE SIDEWALK
400.020 CAST IRON, SQUARE FRAME WITH SQUARE FLAT GRATE FOR CATCH BASINS, HERRING BONE OPENINGS
401.010 CAST IRON, SQUARE FRAME WITH CIRCULAR CLOSED OR OPEN COVER FOR MAINTENANCE HOLES
401.020 CAST IRON, CIRCULAR FRAME WITH CIRCULAR 745MM COVER FOR MAINTENANCE HOLES
403.010 GALVANIZED STEEL, HONEYCOMB GRATING FOR DITCH INLETS
600.040 CONCRETE BARRIER CURB WITH STANDARD GUTTER
604.010 90 DEGREE CONCRETE OUTLET FOR CONCRETE CURB AND GUTTER
605.010 45 DEGREE CONCRETE OUTLET FOR CONCRETE CURB AND GUTTER
701.010 PRECAST CONCRETE MAINTENANCE HOLE, 1200MM DIAMETER
701.030 PRECAST CONCRETE MAINTENANCE HOLE COMPONENTS, 1200MM DIAMETER, TAPERED TOP AND FLAT CAP
702.040 PRECAST CONCRETE DITCH INLET MAINTENANCE HOLE-TYPE A, 1200X1200 MM
705.010 PRECAST CONCRETE CATCH BASIN, 600X600 MM
703.021 PRECAST CONCRETE TWIN INLET FLAT CAP
802.010 FLEXIBLE PIPE, EMBEDMENT AND BACKFILL, EARTH EXCAVATION
803.031 FROST TREATMENT- PIPE CULVERTS, FROST PENETRATION LINE BETWEEN TOP OF PIPE AND BEDDING GRADE
804.030 CONCRETE HEADWALL FOR PIPE LESS THAN 900MM DIAMETER

FOLKARD LANE DEVELOPMENT

FOLKARD LANE, PICTON, K0K 2T0
PRINCE EDWARD COUNTY, ON

DETAILS

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Jp2g PROJECT No.: 25-5035A

NORTH	CLIENT No.:
	DRAFTED: A. HICKS
	DESIGNED: R. ASSELS
	REVIEWED: K. HAWLEY
	APPROVED: K. HAWLEY

D1

Appendix B

Sanitary Design Sheet

Page 1 of 1

Roughness Coefficient (n): 0.013
Development Area: 3.2 ha

n: roughness coefficient

[illegible]